# Review of the contact pressure model for evaluation of the penetration tube ejection from a reactor lower head during a severe accident 

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## 1. Introduction

When damaged core materials are relocated to the reactor lower head with penetrations during a severe accident, the loss integrity of the reactor pressure vessel caused by this type of penetration failure will occur earlier than the global failure of the reactor lower head. Tube ejection out of the reactor lower head is the representative penetration tube failure models suggested by NUREG/CR-5642 [1]. When the penetration tube is locked at the contact interface between the tube and the reactor lower head by interference, the frictional shear force at the interface, which is determined by the contact pressure, should be evaluated in order to assess the tube ejection failure. In this study, the previous contact pressure model [1] is reviewed and the model is improved by considering the deformation of the reactor lower head.

## 2. Review of the previous model

The applied force to reactor lower head and the interface between penetration tube and hole in the reactor lower head by RCS internal pressure is shown in Fig. 1. The ejection of a penetration tube can be resisted by high friction force at the interface of tube and hole. If the weld is failed and the friction force defined by Eq. 1 is less than ejection force by internal pressure, the tube will be ejected.

$$
\begin{equation*}
F_{f}=\int d F_{f}=\int_{0}^{l_{f}} f \cdot P_{s} \cdot 2 \pi r_{o} \cdot d l \tag{1}
\end{equation*}
$$

where $F_{f}$ : friction force,
$f$ : friction coefficient at the interface,
$l_{f}$ : length of contact interface,
$P_{s}$ : contact pressure,
$r_{o}$ : outer radius of tube.
The contact pressure between tube and penetration hole $P_{s}$ is evaluated by Eqs. 2 and 3.

$$
\begin{align*}
& \delta<0, P_{s}=\text { lesser of }\left\{\begin{array}{c}
\frac{\delta \cdot E\left(r_{r}^{2}-r_{i}^{2}\right)}{r_{r}\left[r_{o}^{2}\left(1-2 v_{t}\right)+r_{i}^{2}\left(1+v_{t}\right)\right.} \\
\frac{2}{\sqrt{3}} \sigma_{u} \ln \left(\frac{r_{o}}{r_{i}}\right)
\end{array}\right.  \tag{2}\\
& \delta \geq 0, P_{s}=0 \tag{3}
\end{align*}
$$

where $\delta$ : difference of displacement between tube and hole.


Fig. 1. Schematic of a mechanical load at a penetration area

## 3. Derivation of the contact pressure

For a single hollow cylinder describing a penetration tube, force equilibrium equation of a volume element can be expressed as Eq. 4.


Fig. 2. Cross-section of one hollow tube

$$
\begin{equation*}
\frac{\sigma_{r}-\sigma_{\theta}}{r}+\frac{d \sigma_{r}}{d r}=0 \tag{4}
\end{equation*}
$$

where $\sigma_{r}$ and $\sigma_{\theta}$ : radial and circumferential stress.
For a hollow cylinder whose inner radius is $r_{a}$ and outer radius is $r_{b}$, the following boundary conditions of $\sigma_{r}\left(r_{a}\right)=P_{a}$ and $u\left(r_{b}\right)=u_{b}$ are applied. Then, $\sigma_{r}$ becomes
$\sigma_{r}=\frac{E}{(1+v) r_{a}^{2}+(1-v-2 v m) r_{b}^{2}}\left\{u_{b} r_{b}\left(1-\frac{r_{a}^{2}}{r^{2}}\right)+\frac{P_{a} r_{a}^{2}}{E}[(1+\right.$
$\left.\left.v)+(1-v-2 v m) \frac{r_{b}^{2}}{r^{2}}\right]\right\}$
The pressure at the outer surface $P_{b}$, which is $\sigma_{r}$ in Eq. 24 at $r=r_{b}$, becomes
$P_{b}=\frac{E}{(1+v) r_{a}^{2}+(1-v-2 v m) r_{b}^{2}}\left[u_{b} r_{b}\left(1-\frac{r_{a}^{2}}{r_{b}^{2}}\right)+\frac{2(1-v m) P_{a} r_{a}^{2}}{E}\right]$

If $P_{a}$ is negligibly small and $m=\frac{1}{2}$, Eq. 6 becomes

$$
\begin{equation*}
P_{b}=\frac{E u_{b}\left(r_{b}^{2}-r_{a}^{2}\right)}{r_{b}\left[(1+v) r_{a}^{2}+(1-v) r_{b}^{2}\right]} \tag{7}
\end{equation*}
$$

The contact pressure of NUREG/CR-5642 in the elastic region equals Eq. 7 when $u_{b}=\delta, r_{a}=r_{i}$ and $r_{b}=r_{o}$.

If the material behaves perfectly plastic, the following maximum and minimum principal stress can be considered for a hollow cylinder.

- Case 1: $\sigma_{\theta}$ and $\sigma_{r}$ are maximum and minimum stre sses.
- Case 2: $\sigma_{\theta}$ and $\sigma_{z}$ are maximum and minimum stre sses.
where $\sigma_{r}, \sigma_{\theta}$, and $\sigma_{z}$ are radial, hoop and axial stress, respectively.

For the plane strain or free-closed ends conditions, it satisfies the Tresca yield criterion. Especially for the free-closed ends condition, it satisfies the von Mises yield criterion, too [2]. Generally, the relationship between the maximum and minimum principal stress is expressed by this equation.

$$
\begin{equation*}
\sigma_{\theta}-\sigma_{r}=k \sigma_{Y} \tag{8}
\end{equation*}
$$

where $\quad k=1$ (for the Tresca yield criterion)
$k=\frac{2}{\sqrt{3}}$ (for the von Mises yield criterion)
$\sigma_{Y}$ : yield strength
For Case 2, it only satisfies the following Tresca yield criterion.

$$
\begin{equation*}
\sigma_{\theta}-\sigma_{z}=\sigma_{Y} \tag{9}
\end{equation*}
$$

Because $\sigma_{z}=0$ for the plane stress condition, Eq. 3 becomes

$$
\begin{equation*}
\sigma_{\theta}=\sigma_{Y} \tag{10}
\end{equation*}
$$

By assuming the radial and hoop stress as $\sigma_{r e}$ and $\sigma_{\theta e}$ in the elastic region, and $\sigma_{r p}$ and $\sigma_{\theta p}$ as in the plastic region, the analytical solution for the each stress component can be derived as follows:

## (For Case 1):

By substituting Eq. 2 into the equilibrium equation and using integration, $\sigma_{r p}$ is expressed by Eq. 5 .

$$
\begin{align*}
& \frac{\sigma_{r p}-\sigma_{\theta p}}{r}+\frac{d \sigma_{r p}}{d r}=0 \\
& \rightarrow \sigma_{r p}=k \sigma_{Y} \ln r+C_{1} \tag{11}
\end{align*}
$$

where $r$ and $C_{1}$ are radius and the integral constant.
$\sigma_{\theta e}$ and $\sigma_{r e}$ are expressed in Eqs. $12-13$ by the analytical solution of the equilibrium equation about the radial displacement $u$ in the elastic region.
$\sigma_{r e}=\frac{E}{(1+v)(1-v-2 v m)}\left[(1+v) C_{2}-(1-v-\right.$
2vm) $\frac{1}{r^{2}} C_{3}$ ]
$\sigma_{\theta e}=\frac{E}{(1+v)(1-v-2 v m)}\left[(1+v) C_{2}+(1-v-\right.$
$2 \mathrm{vm}) \frac{1}{r^{2}} C_{3}$ ]
where $E, v$, and $m$ are the elastic modulus, Poisson's ratio, and indicator for the stress and strain condition ( $v$, 0 and $\frac{1}{2}$ for the plane strain, plain stress and free-closed ends, respectively).

The radial displacement $u$ in the plastic region is expressed in Eq. 14.

$$
\begin{equation*}
u=C_{4} r+C_{5} \frac{1}{r} \tag{14}
\end{equation*}
$$

where $C_{4}$ and $C_{5}$ are the integral constants.
By applying the internal pressure $P_{a}$ and displacement $u_{b}$ at the external surface for a hollow cylinder with the inner and outer radius $r_{a}$ and $r_{b}$, respectively, the boundary conditions which yield starts at $r=r^{*}$ are as follows:

$$
\begin{align*}
& r=r_{a}, \quad \sigma_{r p}=P_{a} \\
& r=r_{b}, u=u_{b} \\
& r=r^{*}, \quad \sigma_{\theta e}-\sigma_{r e}=k \sigma_{Y} \\
& r=r^{*}, \quad \sigma_{r e}=\sigma_{r p} \tag{15}
\end{align*}
$$

According to Eqs. $11-14, P_{a}, P_{b}$ and $u_{b}$ become
$u_{b}=\frac{(1-v-2 v m) r_{b}}{E}\left(k \sigma_{Y} \ln \frac{r^{*}}{r_{a}}+P_{a}\right)+\frac{k \sigma_{Y} r_{b}}{2 E}[(1+$
v) $\left.\frac{r^{* 2}}{r_{b}^{2}}+(1-v-2 v m)\right]$

Therefore, $\sigma_{r e}, \sigma_{\theta e}, \sigma_{r p}$, and $\sigma_{\theta p}$ becomes
$\sigma_{r e}=\frac{E}{(1-v-2 v m)}\left\{\frac{u_{b}}{r_{b}}-\frac{k \sigma_{Y} r^{* 2}}{2 E}\left[\frac{(1+v)}{r_{b}^{2}}+\frac{(1-v-2 v m)}{r^{2}}\right]\right\}$

By using Eqs. 17 and 19, the external pressure $P_{b}$ at $r=$ $r_{b}$ for a cylinder at the partially plastic state and fully plastic state is given by the following relation.
(Partially plastic state, $r_{a} \leq r^{*} \leq r_{b}$ ):
$P_{b}=\sigma_{r e}\left(r_{b}\right)=\frac{E}{(1-v-2 v m)}\left[\frac{u_{b}}{r_{b}}-\frac{(1-v m) k \sigma_{Y}}{E} \frac{r^{* 2}}{r_{b}^{2}}\right]$
(Fully plastic state, $r^{*}=r_{b}$ ):
$P_{b}=\sigma_{r p}\left(r_{b}\right)=k \sigma_{Y} \ln \frac{r}{r_{a}}+P_{a}$

## (For Case 2):

By substituting Eq. 10 into the equilibrium equation and using integration, $\sigma_{r p}$ is expressed by Eq. 23.

$$
\begin{align*}
& \frac{\sigma_{r p}-\sigma_{\theta p}}{r}+\frac{d \sigma_{r p}}{d r}=0 \\
& \rightarrow \sigma_{r p}=\sigma_{Y}+\frac{c_{1}}{r} \tag{23}
\end{align*}
$$

$\sigma_{r e}, \sigma_{\theta e}$ and $u$ are obtained by substituting $m=0$ into Eqs. $6-8$. The boundary conditions are same with the Case 1 except $\sigma_{\theta e}$ at $r=r^{*}$. The changed boundary condition is given in Eq. 23.

$$
\begin{equation*}
r=r^{*}, \quad \sigma_{\theta e}=\sigma_{Y} \tag{24}
\end{equation*}
$$

By applying boundary conditions into Eq. 14, $u_{b}$ becomes
$u_{b}=\frac{r_{b}}{E}\left[(1-v)\left(1-\frac{r_{a}}{2 r^{*}}\right)+\frac{(1+v) r^{*} r_{a}}{2 r_{b}^{2}}\right] \sigma_{Y}+$
$\frac{r_{a} r_{b}}{2 E}\left[\frac{(1-v)}{r^{*}}-\frac{(1+v) r^{*}}{r_{b}^{2}}\right] P_{a}$
Therefore, $\sigma_{r e}, \sigma_{\theta e}, \sigma_{r p}$, and $\sigma_{\theta p}$ becomes
$\sigma_{r e}=\frac{E}{(1-v)}\left\{\frac{u_{b}}{r_{b}}-\frac{r^{*} r_{a}\left(\sigma_{Y}-P_{a}\right)}{2 E}\left[\frac{(1+v)}{r_{b}^{2}}+\frac{(1-v)}{r^{2}}\right]\right\}$
$\sigma_{\theta e}=\frac{E}{(1-v)}\left\{\frac{u_{b}}{r_{b}}-\frac{r^{*} r_{a}\left(\sigma_{Y}-P_{a}\right)}{2 E}\left[\frac{(1+v)}{r_{b}^{2}}-\frac{(1-v)}{r^{2}}\right]\right\}$
$\sigma_{r p}=\left(1-\frac{r_{a}}{r}\right) \sigma_{Y}+\frac{r_{a}}{r} P_{a}$
$\sigma_{\theta p}=\sigma_{Y}$
By using Eqs. 26 and 28, the external pressure $P_{b}$ at $r=$ $r_{b}$ for a cylinder at the partially plastic state and fully plastic state is given by the following relation.
(Partially plastic state, $r_{a} \leq r^{*} \leq r_{b}$ ):
$P_{b}=\frac{E}{(1-v)}\left[\frac{u_{b}}{r_{b}}-\frac{r^{*} r_{a}\left(\sigma_{Y}-P_{a}\right)}{E r_{b}^{2}}\right]$
(Fully plastic state, $r^{*}=r_{b}$ ):
$P_{b}=\sigma_{Y}-\frac{r_{a}}{r_{b}}\left(\sigma_{Y}-P_{a}\right)$
If $P_{b}$ corresponds to $P_{s}$ in the lower part of Eq. 2, $P_{b}$ is identical to $P_{s}$ in the lower part of Eq. 2 at $k=\frac{2}{\sqrt{3}}, \sigma_{y}=$ $\sigma_{u}, r_{a}=r_{i}, r_{b}=r_{o}$ and $P_{a}=0$. Therefore, the
assumptions of the previous contact pressure relation in NUREG/CR-5642 can be summarized as follows:

- Assumption 1: negligible internal pressure $\left(P_{a}=0\right)$
- Assumption 2: free-closed ends $\left(\sigma_{z}=\frac{1}{2}\left(\sigma_{r}+\sigma_{\theta}\right)\right)$
- Assumption 3: one hollow cylinder configuration
- Assumption 4: a cylinder in fully plastic state
- Assumption 5: application of the von Mises yield criterion $\left(k=\frac{2}{\sqrt{3}}\right)$
- Assumption 6: evaluation of pressure at rupture by using $\sigma_{u}$ instead of $\sigma_{Y}$


## 4. Validation

Two-dimensional axisymmetric finite element models of the ICI penetration tube using ANSYS Mechanical R18.0 are constructed in order to validate the derived assumptions of the contact pressure model. PLANE183, an 8 -node element with mid-nodes, is selected for meshing. Half of the ICI penetration tube in the length direction is modeled using a symmetry condition. The geometry according to the stress and strain conditions with the dimension in Table 1 are shown in Fig. 3. In the free-closed ends condition shown in Fig. 3, a physical cap part at the upper surface of the axisymmetric tube is considered, and its outer surface is loaded by pressure corresponding to the contact pressure by $\delta$. The geometrical nonlinearity is not considered because the derived equations in Chap. 3 assumes no changes of the geometrical domain. The $\sigma_{r}$ according to $r$ and $\delta$ at 400 K with the free-closed end condition is shown in Fig. 4 7. The $\sigma_{r}$ with von Mises yield criterion by Eqs. 17 and 19 follows the FEM result very well

Table 1. Geometry of the ICI penetration tube for finite element modeling

| Inner radius $\left(r_{a}\right)$ | 9.5 mm |
| :--- | :--- |
| Outer radius $\left(r_{b}\right)$ | 38.1 mm |
| Length | 175 mm |



Fig. 3. Axisymmetric geometry according to the stress and strain conditions (one hollow cylinder)


Fig. 4. $\sigma_{r}$ according to $r$ at 400 K with the free-closed end condition and $\delta=(0.01-0.03) \mathrm{mm}$


Fig. 5. $\sigma_{r}$ according to $r$ at 400 K with the free-closed end condition and $\delta=(0.04-0.06) \mathrm{mm}$


Fig. 6. $\sigma_{r}$ according to $r$ at 400 K with the free-closed end condition and $\delta=(0.07-0.09) \mathrm{mm}$


Fig. 7. $\sigma_{r}$ according to $r$ at 400 K with the free-closed end condition and $\delta=(0.1-0.3) \mathrm{mm}$


Fig. 8. $\sigma_{r}$ according to $r$ at 400 K with the free-closed end condition and $\delta=(0.4-0.5) \mathrm{mm}$

## 5. Conclusions

In this study, the contact pressure evaluation method from NUREG/CR-5642 has been reviewed. The assumptions of a geometrical configuration and the 'stress and strain condition' of the previous model in NUREG/CR-5642 are derived in a reversed manner by a stress analysis. As a result, it was found that the previous model only considers the deformation of the ICI penetration tube itself and it assumes the free-closed ends condition. However, these assumptions cannot explain the deformation of the reactor lower head hole. And more general stress and strain conditions are needed to explain the mounting condition of the penetration tube. And the previous model considers the rupture pressure by using the ultimate strength instead of the yield strength for the analytical stress analysis which results in overestimated contact pressure. These drawbacks can be modified by choosing the configuration of two concentric hollow cylinders which represent the ICI penetration tube and the reactor lower head with a generalized expression of the stress and strain conditions and by using the yield strength.

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## REFERENCES

[1] J. L. Rempe et al., Light Water Reactor Lower Head Failure Analysis, NUREG/CR-5642, 1993.

